

January 27, 2026

S252168

Kingsdale Development
95 Royal Crest Ct., Unit 21-22
Markham, ON L3R 9X5

Attn: Isaac de Ceuster

**Re: Servicing Feasibility Study
Proposed 10-Storey Apartment Building Development
119 & 127 Centre St, London, Ontario**

1. INTRODUCTION

This Servicing Feasibility Study (Study) has been prepared by Strik, Baldinelli, Moniz Ltd. (SBM) to address the servicing feasibility for the proposed residential development located at 119 & 127 Centre Street, London, Ontario. This Study is intended to represent a component of the overall application package and should be read in conjunction with all other submitted documents.

The subject site abuts the Centre Street Right-Of-Way (ROW) to the north, and residential lands to the east, west and south. The subject site is currently comprised of a residential dwelling with associated driveways, garage, and accessory structures, and is approximately 0.42 ha in area. The proposed development consists of a 10-storey, 156-unit apartment building with associated surface/underground parking and landscape areas.

This Study is to determine the adequacy of the existing City services in support of the Zoning By-law Amendment (ZBA) application for the proposed development.

Design requirements have been based on the City of London Design Specifications & Requirements Manual (DS&RM), updated January 2025, and the current edition of the Ontario Building Code (OBC).

2. SANITARY SERVICING

As per the City of London's Record of Pre-Application Consultation comments dated July 17, 2025, and the City's record drawing 6596 dated August 1969, appended to this Study, the proposed development is intended to outlet to the existing 350 mm sanitary sewer located within the sanitary easement east of 119 Centre Street.

Building service sizing calculations, in accordance with the Ontario Building Code (OBC), will be completed during the detailed design stage for Site Plan Approval (SPA) process.

The proposed sanitary flows from the subject site are shown on the Sanitary Sewer Design Sheet appended to this Study. Using a per capita sanitary flow of 230 L/capita/day in accordance with the DS&RM, and an estimated site population of 250 people (156 units at 1.6 people per unit), the anticipated peak sanitary flow for the proposed development is 3.01 L/s. This value reflects the use of an uncertainty factor of 1.1 and Harmon Peaking Factor of 4.11. When combined with the infiltration allowance of 0.03 L/s (8640 L/ha/day as per DS&RM Section

3.8.1), the resulting peak sanitary flow is 3.04 L/s. Refer to the Sanitary Sewer Design Sheet attached in Appendix B.

Section 3.16.2.(b) of the DS&RM (2025) requires a minimum 150 mm diameter sanitary private drain connection (PDC) at a minimum slope of 1.0% to service a residential multi-family block. A new sanitary PDC will be designed during the SPA stage.

A sanitary sewer capacity analysis was conducted downstream of the subject site to the terminal point of the 375 mm diameter sanitary sewer on Base Line Road West at maintenance hole (MH) WT217 to confirm adequate residual capacity with the addition of the proposed development. The analysis included all contributing upstream residential and commercial lands along Centre Street and the sanitary easement east of 119 Centre Street.

In addition to the design parameters indicated above, an institutional density of 100 people per hectare was used at 86 Base Line Road, while residential densities of 150 units per hectare were applied to 55 Centre Street and 106 & 112 Base Line Road West, based on applicable zoning designations. Due to the lack of existing drainage area mapping for Centre Street and Base Line Road West, tributary areas were delineated by SBM using service connections identified on available as-built drawings and measured in Bluebeam.

With the inclusion of sanitary flows generated by the subject site, no downstream sanitary segments were identified as operating at or above capacity. The sewer segment extending from MH WT2364 to WT217 was calculated to have a utilized capacity of 23.5%.

Please see the sanitary sewer design sheets and sanitary drainage areas attached in Appendix B for additional details, along with relevant as-built drawings provided in Appendix A.

3. STORM SERVICING AND STORMWATER MANAGEMENT

3.1 Pre-Development Conditions

Pre-development conditions were obtained from satellite imagery and will be furthered verified by the topographical survey to be completed at the time of detailed design. Under pre-development conditions, the subject site consists of existing residential dwelling/sheds/detached garages (approx. 533.14 m²) with associated gravel parking/ driveways (817.11 m²), and the remainder being landscaped/open space (2,786.33 m²). Resulting in an existing runoff coefficient (C-value) of 0.43.

As per the City's as-constructed drawing 11755A dated August 1995, and the Record of Pre-Application Consultation comments dated July 17, 2025, 119 & 127 Centre Street (Area A101) is tributary to the existing 450 mm storm sewer in the Centre Road ROW, which has an allowable runoff coefficient (C-value) of 0.65.

3.2 Post-Development Conditions

Post-development conditions were interpreted from the Site Plan, prepared by Siv-ik Planning and Design Inc., dated June 26, 2025, provided in Appendix A.

Under post-development conditions the subject site will consist of a 10-storey, 156-unit apartment building (724.92 m²) with associated asphalt/ concrete parking surfaces (1,971.96 m²) and landscaped areas (1,439.69 m²). The resulting runoff coefficient is 0.66, as per SWM calculations in Appendix C. As the C-value in post-development conditions exceeds the allowable C-value in pre-development conditions, stormwater management (SWM) quantity controls will be implemented to restrict the flows from post-development to the allowable rates for the 2-to-100-year rainfall events.

The Preliminary Stormwater Management Calculations, provided in Appendix C, indicate approximate required storage volumes of 20.74 m³ and 60.75 m³ during the 2-year and 100-year rainfall events respectively. A combination of methods will be considered to meet the storage requirements including temporary surface

ponding on asphalt, flat rooftops, and landscape areas; and sub-surface storage within storm sewers and structures. Low impact development (LID) will be considered depending on soil conditions. Where required, underground chambers will also be incorporated as an effective means of providing additional storage within the site's constraints. Detailed SWM calculations and grading design will be prepared at the time of detailed design for SPA.

3.3 *Water Quality*

As per Section 6.2.1 of the DS&RM and the City of London Record of Pre-Application Consultation comments dated July 17, 2025, the proposed development is tributary to the Thames River watershed and is therefore subject to municipal stormwater quality control requirements. The applicable water quality target for the site is a minimum long-term average removal of 70% Total Suspended Solids (TSS).

Stormwater quality control measures will be designed to meet this target at the detailed design stage. Supporting calculations and design details will be provided as part of the SPA submission.

4. SUMMARY

Based on the above, the existing municipal sanitary and storm servicing infrastructure are considered to have sufficient residual capacity to support the proposed 156-unit apartment development at 119 & 127 Centre Street, London.

5. LIMITATIONS

This Study was prepared by SBM for Kingsdale Development Inc. and the City of London. Use of this Study by any third party, or any reliance upon its findings, is solely the responsibility of that party. SBM accepts no responsibility for damages, if any, suffered by a third party as a result of decisions made or actions undertaken as a result of this study. Third party use of this Study, without the express written consent of the Consultant, denies any claims, whether in contract, tort, and/or any other cause of action in law, against the consultant.

All findings and conclusions presented in this Study are based on site conditions as they appeared in the information presented to SBM and related to in this document. This Study is not intended to be exhaustive in scope, or to imply a risk-free development. It should be recognized that the passage of time may alter the opinions, conclusions, and recommendations provided herein, as well as any changes in the layout of the development.

The design was limited to the documents referenced herein and SBM accepts no responsibility for the accuracy of the information provided by others. All designs and recommendations presented in this Study are based on the information available at the time of the review.

This document is deemed to be the intellectual property of SBM in accordance with Canadian copyright law.

6. CLOSURE

We trust this Brief meets your satisfaction. Should you have any questions or require further information, please do not hesitate to contact us.

Respectfully submitted,

Strik, Baldinelli, Moniz Ltd.

Planning • Civil • Structural • Mechanical • Electrical



Ben Hyland, P.Eng., PMP
Civil Team Lead, Eng IV
Associate I



Matthew Galopoulos
Civil Engineering Trainee I

List of Appendices

- Appendix A: Site Plan prepared by Siv-ik Planning and Design Inc., dated June 26, 2025
 City of London Record Drawing 6596 prepared by the City of London, dated August 1969
 City of London Record Drawing 11755A prepared by Development Engineering, dated August 1995
- Appendix B: Sanitary Sewer Capacity Assessment prepared by SBM
 Sanitary Sewer Capacity Assessment Area Markup
- Appendix C: Stormwater Management Calculations – Preliminary Storage Requirements

APPENDIX A

Site Plan prepared by Siv-ik Planning and Design Inc., dated June 26, 2025

City of London Record Drawing 6596 prepared by the City of London, dated August 1969

City of London Record Drawing 11755A prepared by the City of London, dated August 1995

CENTRE STREET

66.5M

EXTENT OF U/G PARKING

PROPOSED APARTMENT BUILDING

HEIGHT: 10 STOREYS
DWELLING UNITS: 156

10.4M

3.1M

12.0M

6.7M
2.3M
6.0M

3.5M

12.0M

VEHICLE LAYBY

3.5M

2.7M

6

6.7M

2.7M

5.5M

1

5.5M

2.4M

3.4M

5

1.5M

15

16

(21)

U/G PARKING
ENTRANCE

DOWN

WASTE
COLLECTION
POINT

5.7M

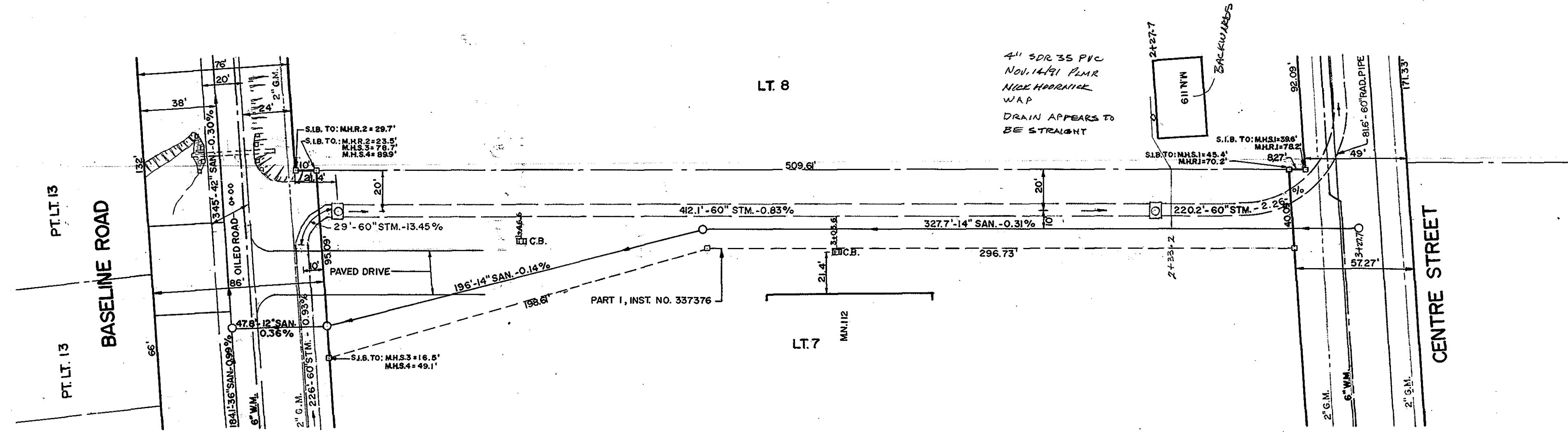
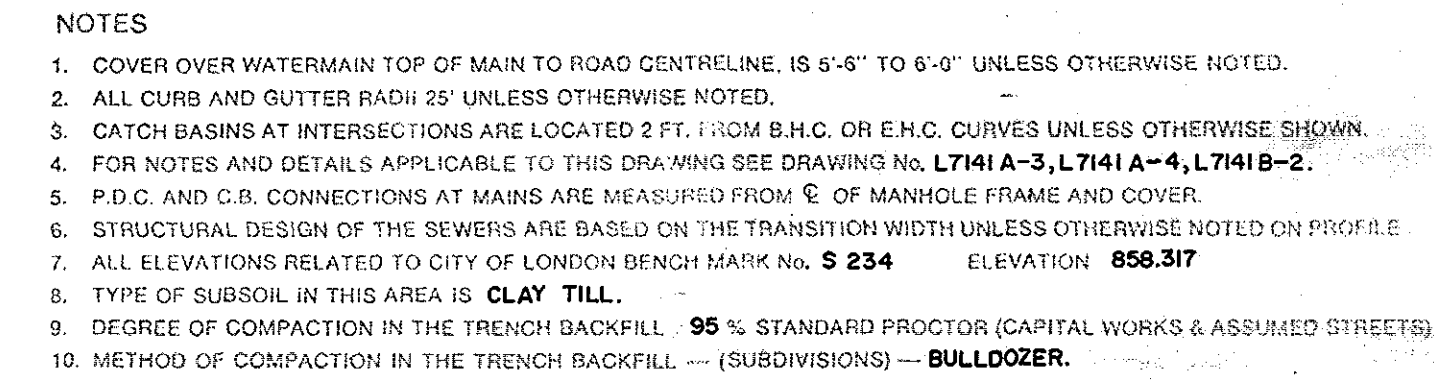
COMMON
AMENITY
AREA

6.4M

66.5M

60.2M

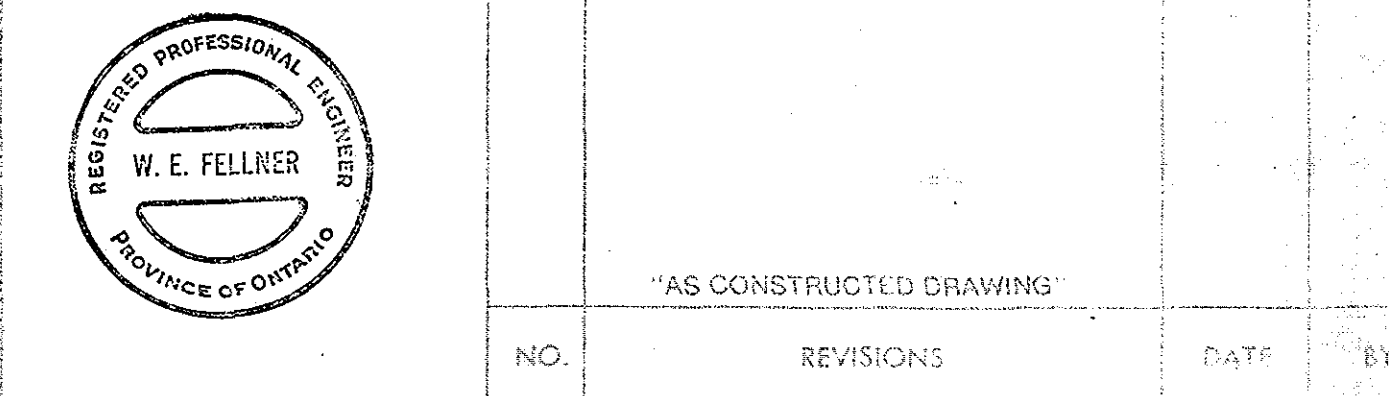
60.2M



P.D.C.s AND WATER SERVICES ARE INSTALLED IN STANDARD LOCATIONS AS ABOVE UNLESS OTHERWISE SHOWN AND NOTED.

	SIZE	STRENGTH	MAT'L	JOINT	BEDDING	MANUFACTURER
SAN. P.D.C.s	4"	1500	A.C.	R.G.	C	JOHN MANVILLE
ST. P.D.C.s	4"	1500	A.C.	R.G.	C	
C.B. CONNECTIONS	8"	E.S.	CONC.	R.G.	C	
PRE-CAST CONC. M.H.	48"	3000	CONC.	P.L.C.	N/A	

SERVICES	COMPLETION	CONTRACTOR
SANITARY SEWERS P.D.C.'s & M.H.'s	APRIL 1972	D.L. COATSWORTH
STORM-SEWERS, P.D.C.'s, M.H.'s & C.B.'s	AUGUST 1969	FRAMAT CONST.
WATERMANS & WATER SERVICES		
GRANULAR ROAD BASE		
CURB & GUTTER		
SIDEWALKS		
PAVING		

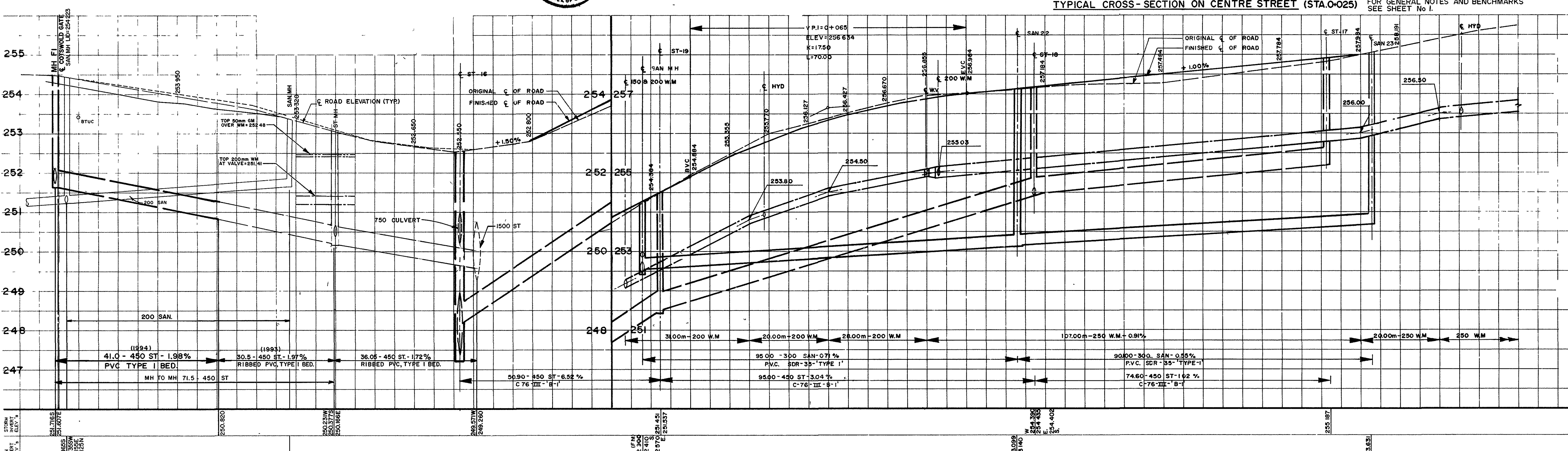
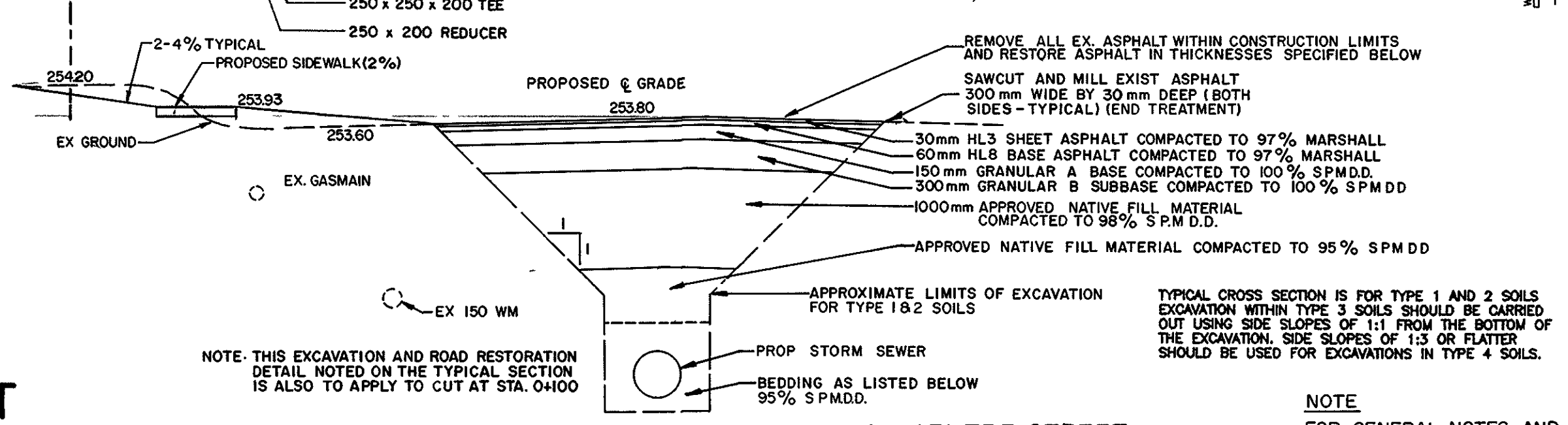
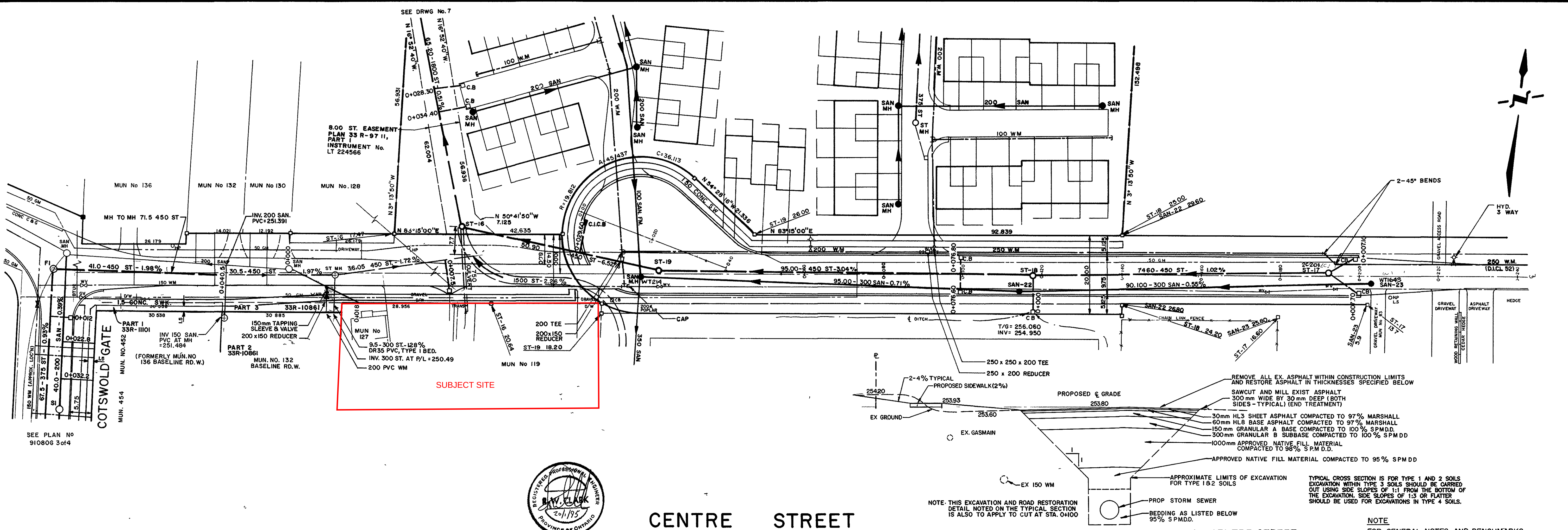


L.E. MACDOUGAL & W. HARTLING

CENTRE STREET
FROM EASEMENT TO OUTLET

DESIGN BY RE.FELLNER DRAWN BY J.D.M. CHECKED BY W.E.F.	FIELD BOOK NO: 53, 54A, 8, 54B SCALE : HORIZ: 1"=40' , VERT: 1"=4' DATE
	CITY ENGINEER'S DEPARTMENT PROJECT NO. REGION HEAD DRAWING NO.
DEVELOPMENT ENGINEERING (LONDON) LIMITED 650 WATERLOO ST., SUITE 'A' LONDON ONTARIO	APPROVED BY PROJECT NO. 6596 DRAWING NO.
PROJECT NO.: L714/A DRAWING NO.: 1	CITY ENGINEER

6596



AS CONSTRUCTED NOTES		AS CONSTRUCTED SERVICES		COMPLETION	DESIGN	DATE	BY
1. SEE DRAWING NO. 910806 3 of 4 FOR FURTHER DETAIL		SANITARY SEWERS, PDC's & MH's		JUNE 1994	DESIGN RWC	AUG 1995	CW
2. SEWER DESIGN, TRANSITION WIDTH OR AS NOTED		STORM SEWERS, PDC's, CB's & MH's		JUNE 1994	DRAWN UB GC		
3. REFERENCE S.M. No. 8-228 ELEVATION		W.S.C.'s		JUNE 1994	CHECKED LGWS		
		GRANULAR BASE		JULY 1994	APPROVED		
		CURB & GUTTER		SEPT. 1994	DATE		
		SIDEWALKS		SEPT. 1994			
		PAVING - BASE		SEPT. 1994			
		- SURFACE		JULY 1995			

REVISIONS		DATE	BY
1	AS CONSTRUCTED	AUG 1995	CW

FOR STORM SEWER WEST OF M.N. 136 BASELINE RD. ONLY.

ENGR'S STAMP

REGISTERED PROFESSIONAL ENGINEER

R.W. CLARK

8/9/93

PROVINCIAL C.

development engineering

(London) Limited

PROJECT ENGINEER

CORPORATION OF THE CITY OF LONDON

DIVISION HEAD

CITY ENGINEER

SCALE

H = 1:500

V = 1:50

50m 0 100m

Horizontal

0.5m 0 1.0m

Vertical

TITLE

GALAXY HOMES

STORM SEWER OUTLET - MN 136 BASELINE ROAD WEST

CENTRE STREET

FROM COTSWOLD GATE

TO 370m EAST OF COTSWOLD GATE

PROJECT No

91080 G

SHEET No

2 OF 4

PLAN FILE No

11,755 A

APPENDIX B

Sanitary Sewer Capacity Assessment prepared by SBM

Sanitary Sewer Capacity Assessment Area Markup

sbm

STRIK
BALDINELLI
MONIZ

PLANNING • CIVIL • STRUCTURAL • MECHANICAL • ELECTRICAL

q = average per capita daily flow
i = unit of industrial flow
ic = unit of peak extraneous flow
High Density Residential (Apartment Buildings)
Medium Density Residential (Multi-Family/Townhouse)
Low Density Residential (Single Family/Semi-Detached)
Commercial/Institutional
Uncertainty Factor

230
25.00
0.1
1.60
2.40
3.00
100.00
1.10

L/cap.d
m³/ha.d
L/ha.s
ppu
ppu
ppu
people/ha

P = population
M = peaking factor (Harmon)
P = p x # units / 1000
M = 1 + 14 / (4 + P^{0.13})
Q = (P x q x M) / 86.4
M Min = 4
M Max = 10

Q(p) = peak population flow (L/s)
Q(i) = i x A = peak extraneous flow (L/s)
Q(c) = $\frac{c \times A}{86400}$ = peak commercial flow (L/s)
Q(d) = Q(p) + Q(i) = peak design flow (L/s)

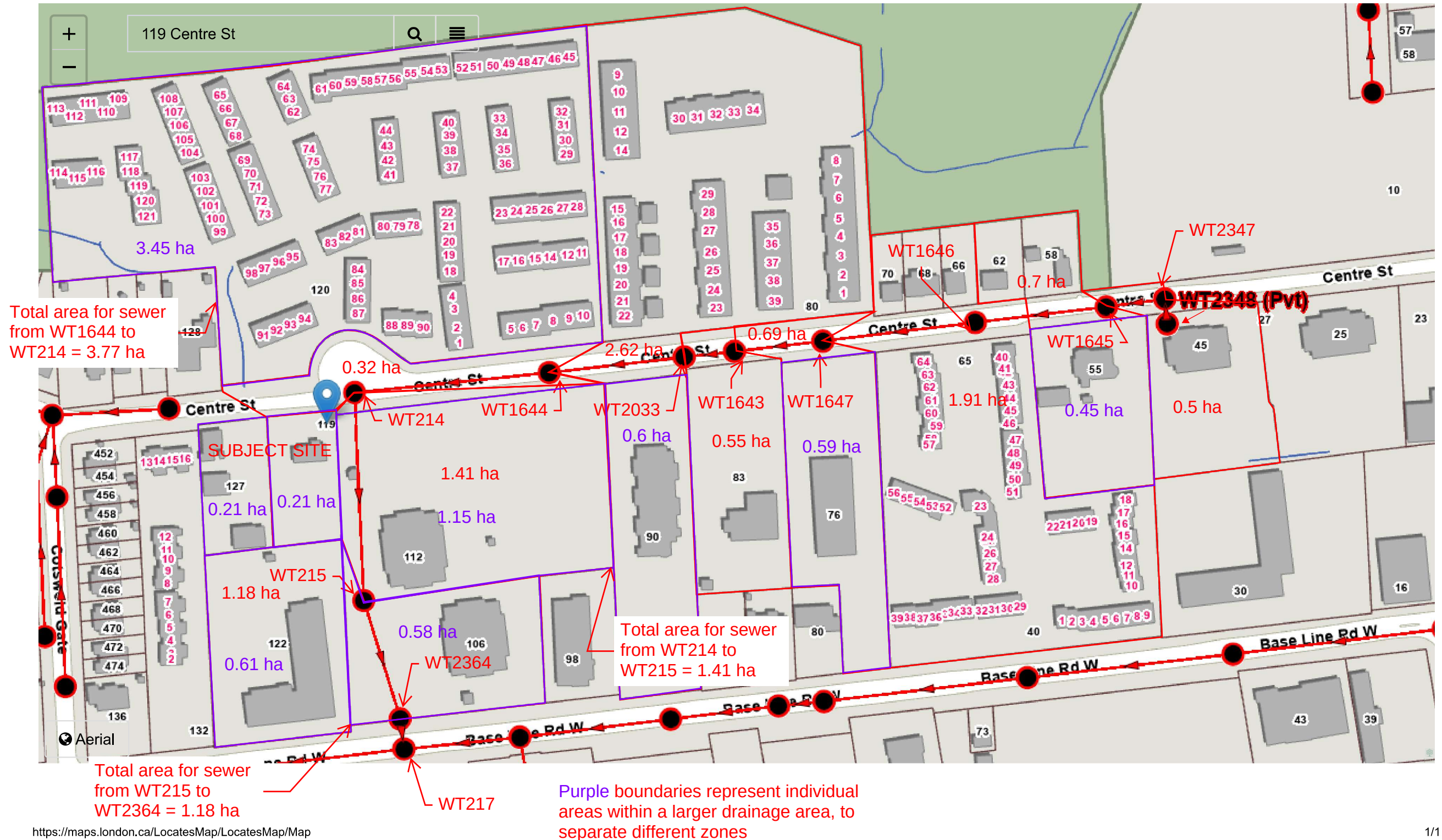
Min full flow velocity = 0.6 m/s

DATE: Jan. 27, 2026
DESIGNED BY: MGA
REVIEWED BY: BH

FILE No.: S252168
PROJECT: 119 & 127 Centre Street, London

LOCATION			INDIVIDUAL								CUMULATIVE					DESIGN FLOWS					SANITARY SEWER							
STREET	FROM MH	TO MH	No. HIGH-DENSITY RESIDENTIAL UNITS	No. MEDIUM-DENSITY RESIDENTIAL UNITS	No. LOW-DENSITY RESIDENTIAL UNITS	RES. POP	COMM. POP.	RES. AREA (ha)	COMM/INST AREA (ha)	INDUSTRIAL AREA (ha)	POP	COMM/INST AREA (ha)	INDUSTRIAL AREA (ha)	RES. AREA	TOTAL AREA (ha)	PEAKING FACTOR (M)	POPULATION FLOW, Q(p) (L/s)	INDUSTRIAL FLOWS (L/s)	PEAK EXTRANEEOUS FLOW, Q(i) (L/s)	PEAK DESIGN FLOW, Q(d) (L/s)	LENGTH (m)	PIPE SIZE DIAMETER (mm)	GRADE (%)	MANNING'S n	CAPACITY (L/s)	FULL FLOW VELOCITY (m/s)	ACTUAL VELOCITY (m/s)	PERCENT FULL
Centre St	WT2348	WT2347	61.00	0.00	0.00	97.6	0.00	0.5	0.00	0.00	97.60	0.00	0.00	0.50	0.50	4.25	1.214	0.000	0.050	1.264	28.0	250	0.30%	0.013	32.6	0.66	0.32	3.9%
Centre St	WT2347	WT1646	68.00	0.00	2.00	114.8	0.00	0.7	0.00	0.00	212.40	0.00	0.00	1.20	1.20	4.14	2.574	0.000	0.120	2.694	63.3	250	0.30%	0.013	32.6	0.66	0.39	8.3%
Centre St	WT1646	WT1647	0.00	64.00	3.00	162.6	0.00	1.91	0.00	0.00	375.00	0.00	0.00	3.11	3.11	4.04	4.431	0.000	0.311	4.742	73.8	250	0.31%	0.013	33.1	0.67	0.46	14.3%
Centre St	WT1647	WT1643	89.00	0.00	0.00	142.4	0.00	0.69	0.00	0.00	517.40	0.00	0.00	3.80	3.80	4.00	6.060	0.000	0.380	6.440	42.8	300	0.52%	0.013	69.7	0.99	0.60	9.2%
Centre St	WT1643	WT2033	0.00	0.00	0.00	0	55.00	0	0.55	0.00	572.40	0.55	0.00	3.80	4.35	4.00	6.705	0.000	0.435	7.140	24.5	300	0.55%	0.013	71.7	1.01	0.63	10.0%
Centre St	WT2033	WT1644	89.00	38.00	0.00	233.6	0.00	2.62	0.00	0.00	806.00	0.55	0.00	6.42	6.97	4.00	9.441	0.000	0.697	10.138	65.6	300	0.55%	0.013	71.7	1.01	0.69	14.1%
Centre St	WT1644	WT214	121.00	0.00	0.00	193.6	0.00	3.77	0.00	0.00	999.60	0.55	0.00	10.19	10.74	4.00	11.708	0.000	1.074	12.782	95.0	300	0.71%	0.013	81.5	1.15	0.80	15.7%
Sanitary Easement (Includes Subject Site)	WT214	WT215	329.00	0.00	0.00	526.4	0.00	1.41	0.00	0.00	1526.00	0.55	0.00	11.60	12.15	4.00	17.874	0.000	1.215	19.089	99.9	350	0.31%	0.013	81.2	0.84	0.66	23.5%
Sanitary Easement	WT215	WT2364	179.00	0.00	0.00	286.4	0.00	1.19	0.00	0.00	1812.40	0.55	0.00	12.79	13.34	4.00	21.229	0.000	1.334	22.563	59.7	350	0.41%	0.013	93.4	0.97	0.76	24.2%
Base Line Rd W	WT2364	WT217	0.00	0.00	0.00	0	0.00	0.21	0.00	0.00	1812.40	0.55	0.00	13.00	13.55	4.00	21.229	0.000	1.355	22.584	14.5	375	0.36%	0.013	105.2	0.95	0.72	21.5%
		TOTALS	936			1757.4	55.00	13.00	0.55	0.00																		

* No service connections found on as-builts, therefore only the ROW area was considered for infiltration
** 150 units/hectare used for Centre Street Mun #65, per R9-7 zoning density, plus the 2 low-density residential dwellings at Centre Street Mun #58 and Mun #62
*** Medium density residential dwellings at Centre Street Mun #65, and the 3 low-density residential dwellings at Centre Street Mun #66, Mun #68, and Mun #70
**** 150 units/hectare used for Centre Street Mun #76, per R9-7 zoning density
***** 150 units/hectare used for Base Line Road W Mun #90, per R9-7 zoning density, plus the 38 medium density residential dwelling units at Centre Street Mun #80
***** 150 units/hectare used for Base Line Road W Mun #112, per R9-7 zoning density, plus the proposed high-density residential dwelling at Centre Street Mun #119
***** 150 units/hectare used for Base Line Road W Mun #106 and Mun #122, per R9-7 zoning density



APPENDIX C

Stormwater Management Calculations – Preliminary Storage Requirements

Stormwater Management Calculations - Preliminary Storage Requirements

DATE:	January 27, 2026
JOB No.:	S252168
Client:	Kingsdale Development
Project:	Proposed 10 Storey Apartment Building
Location:	119 & 127 Centre St, London ON

CITY OF LONDON-3 CHICAGO RAINFALL DISTRIBUTION PARAMETERS*

Return Period (years)	A, B, C Parameters		
	A	B	C
2	754.360	6.011	0.810
5	1183.740	7.641	0.838
10	1574.382	9.025	0.860
25	2019.372	9.824	0.875
50	2270.665	9.984	0.876
100	2619.363	10.500	0.884
250	3048.220	10.030	0.888

*Intensity $i = A / (t + B) \cdot C$ (mm/hr), t = duration (minutes)

*Refer to the City of London Design Specification & Requirements Manual (DS&RM), Section 6.

PRE-DEVELOPMENT CONDITIONS**

** Pre-Development Conditions were obtained from satellite imagery

PRE-DEVELOPMENT OVERALL SITE

	Area (m ²)	C	A*C
Total Area:	4136.58		
Building Area:	533.14	0.9	479.826
Asphalt:	817.11	0.9	735.399
Gravel:	0.00	0.7	0.000
Landscaped/Open:	2786.33	0.2	557.266
Totals:	4136.58		1772.491
$C_{eq} = \text{Sum}(A \cdot C) / \text{Sum}(A) =$	0.43		

2-Year Pre-Development Flows:

C-value =	0.43
**Time of concentration t_c =	21.1 min
Intensity, i (@ t_c) =	52.09 mm/hr
Pre-Development Flow, $Q_p = 2.78 \cdot C \cdot i \cdot A =$	25.67 l/s

130% of 100-Year Pre-Development Flows:

C-value =	0.43
**Time of concentration t_c =	21.1 min
Intensity, i (@ t_c) =	160.85 mm/hr
Pre-Development Flow, $Q_p = 2.78 \cdot C \cdot i \cdot A =$	79.26 l/s

POST-DEVELOPMENT CONDITIONS***

*** Post-Development Conditions were obtained from the Vision Brief prepared by Siv-ik Planning and Design Inc., dated June 26 2025

POST-DEVELOPMENT OVERALL SITE

	Area (m ²)	C	A*C
Total Area:	4136.58		
Building Area:	724.92	0.9	652.431
Asphalt/Concrete:	1971.96	0.9	1774.767
Gravel:	0.00	0.7	0.000
Landscaped/Open:	1439.69	0.2	287.939
Totals:	4136.58		2715.136
$C_{eq} = \text{Sum}(A \cdot C) / \text{Sum}(A) =$	0.66		

2-Year Post-Development Flows:

C-value =	0.66
**Time of concentration t_c =	13.25 min
Intensity, i (@ t_c) =	68.71 mm/hr
Pre-Development Flow, $Q_p = 2.78 \cdot C \cdot i \cdot A =$	51.86 l/s

130% of 100-Year Post-Development Flows:

C-value =	0.66
**Time of concentration t_c =	13.25 min
Intensity, i (@ t_c) =	207.04 mm/hr
Pre-Development Flow, $Q_p = 2.78 \cdot C \cdot i \cdot A =$	156.27 l/s

ATTENUATE POST-DEVELOPMENT FLOWS TO ALLOWABLE PRE-DEVELOPMENT LEVELS:

RETURN PERIOD	ALLOWABLE PRE-DEVELOPMENT FLOWS (L/s)	ALLOWABLE POST-DEVELOPMENT RELEASE (L/s)	ALLOWABLE SURFACE RELEASE (L/s)
2-year	25.67	25.67	0.00
130% of 100-year	79.26	79.26	53.59

PRELIMINARY STORAGE CALCULATIONS

FLOW RESTRICTION CALCULATION

Restricted Flow Rate, Q = 25.67 L/s

***Proposed site release rate to 375.13 L/s (Overall Site + External Area)

RAINFALL DATA

Rainfall Data - London Rainfall Intensity Duration
--

2 Year Storm Event	
Duration (min.)	Intensity "i" (mm/hr)
5	108.07
10	79.80
15	64.03
30	41.39
60	25.33
120	15.01
180	10.95

130% of 100 Year Storm Event	
Duration (min.)	Intensity "i" (mm/hr)
5	301.92
10	235.80
15	194.43
30	129.17
60	79.13
120	45.91
180	32.86

STORAGE CHART

Inflow, Q _i 2.78°C*i*A (L/s)	Volume In Qt*t*60/1000 (m ³)	Restricted Outflow, Q _o (L/s)	Surface Outflow, Q _o (L/s)	Volume Out Q _o *t*60/1000 (m ³)	Difference/ Storage (m ³)
81.57	24.47	25.67	0.00	7.70	16.77
60.23	36.14	25.67	0.00	15.40	20.74
48.33	43.50	25.67	0.00	23.10	20.40
31.24	56.23	25.67	0.00	46.20	10.03
19.12	68.84	19.12	0.00	68.84	0.00
11.33	81.55	11.33	0.00	81.55	0.00
8.26	89.23	8.26	0.00	89.23	0.00
Max. Storage Volume (m ³) =					20.74

Inflow, Q _i 2.78°C*i*A (L/s)	Volume In Qt*t*60/1000 (m ³)	Restricted Outflow, Q _o (L/s)	Surface Outflow, Q _o (L/s)	Volume Out Q _o *t*60/1000 (m ³)	Difference/ Storage (m ³)
227.89	68.37	25.67	53.59	23.78	44.59
177.99	106.79	25.67	53.59	47.55	59.24
146.76	132.08	25.67	53.59	71.33	60.75
97.50	175.49	25.67	53.59	142.66	32.83
59.73	215.02	25.67	34.06	215.02	0.00
34.66	249.52	25.67	8.99	249.52	0.00
24.81	267.90	24.81	0.00	267.90	0.00
Max. Storage Volume (m ³) =					60.75