

Stormwater Management Report For 691 Fanshawe Park Road East

Prepared by:

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PROTOTECK ENGINEERING ASSOCIATES INC.

90 Albert Street
London, Ontario
N6A 1L8

File No.: 24-1011

Date: October 09, 2024



Introduction:

This report is submitted to support the rezoning and development of the property at 691 Fanshawe Park Road East in the City of London. Its primary purpose is to address stormwater management for the proposed development. We aim to ensure that all information is presented in a clear and accessible format, adhering to accessibility guidelines to accommodate all readers.

Property Overview

The subject property is located on the south side of **Fanshawe Park Road East** and is surrounded by **Single-Family Residential Development**. The total lot area is approximately **1369.33 m²** (see Appendix **A** for the existing site layout).

The proposed development consists of two stacked townhouse buildings:

- One **4-unit stacked townhouse**
- One **6-unit stacked townhouse**
There will also be **11 parking spaces** situated between the two townhouse buildings to meet parking requirements (see architectural site plan in Appendix **A**).

Existing Site Conditions

Quantity Control

Currently, the site drains generally towards the south, flowing towards the southern property line. The **elevation drop** from the northern property line to the southwestern corner of the site is approximately **0.29 meters**, with an average slope of **0.54%**.

The site's drainage is divided into two distinct catchments:

- **Catchment 101** directs flows overland towards **Fanshawe Park Road**.
- **Catchment 102** conveys drainage towards the southwest corner and also captures overland flows from external lands to the east.



Catchment Summary:

Catchment Area	Area (m ²)	Runoff Coefficient (C)
101 (Towards Fanshawe Rd)	785.98	0.37
102 (Towards Southwest)	583.35	0.37

Post-Development Conditions

Quantity Control

After development, the existing home, including the asphalt driveway, sidewalks, patio, porch, and shed, will be removed. The site will be redeveloped with the two stacked townhouse buildings and the 11 parking spaces. The total area for both townhouse buildings is approximately **349.85 m²**.

The stormwater runoff coefficient for the site, as per the **City of London's Stormwater Management Plan (Drawing 27101)**, is **C=0.75**. This site is tributary to an existing **825mm storm sewer** (see Appendix B). Therefore, the peak runoff (Qall) for this area is estimated to be **15.7 l/s**.

Post-development catchments are summarized below:

Catchment Summary (Post-Development):

Catchment Area	Area (m ²)	Runoff Coefficient (C)
U1 (Uncontrolled)	99.64	0.33
201 (Controlled)	1000.07	0.85
U2 (Uncontrolled)	269.62	0.20

Catchment **U2** will remain uncontrolled to convey overland flows from the adjacent lands to the east. This catchment has been reduced from the existing **Catchment 102** condition, so additional stormwater management is not required.



Stormwater Management Storage Requirements

The following table summarizes the required storage volumes for the 2-year, 5-year, and 100-year storm events. The storage will be provided on the surface of the parking lot, and will be controlled through an orifice plate in a catch basin.

Storm Event	Required Storage Volume
2-Year Storm	3.33 m ³
5-Year Storm	6.48 m ³
100-Year Storm	18.8 m ³

Please refer to Appendix **A** for detailed SWM calculations and to Appendix **B** for the concept SWM plan.

Quality Control

In accordance with the **City of London Design Specifications & Requirements Manual (Section 6.2.1.3, point a)**, water quality control is required for developments with 30 or more parking spaces. Since the proposed development includes **11 parking spaces**, water quality control is **not required** for this project.

We trust this report meets your requirements. Should you have any questions or need further clarification, please feel free to contact us.

Prototeck Engineering Associates Inc.

Nick Aroutzidis, M.A.Sc., P.Eng.
Principal Consulting Engineer





Notes on Accessibility:

- To ensure compliance with AODA, this report is presented in a clear, structured format with headings, bullet points, and tables for easy navigation.
- If any part of the document requires further assistance or if additional accessibility accommodations are needed, please contact us, and we will be happy to assist.



APPENDIX "A"

PRE-DEVELOPMENT

	Area (m ²)	C	A°C
Total Area:	1036.38		
Building Area:	137.12	0.9	123.41
Driveway	121.93	0.9	109.74
Sidewalk	31.48	0.9	28.33
Front Portch	7.75	0.9	6.98
Patio	22.12	0.9	19.91
Shed	12.55	0.9	11.30
Landscape/Open:	1036.38	0.2	207.28
Totals:	1369.33		506.93
$C_{eq} = \text{Sum}(A^\circ C) / \text{Sum}(A) =$	0.37		

AES PARAMETERS FOR INTENSITY DURATION FREQUENCY CURVES

RETURN PERIOD	A, B, C, PARAMETERS		
	A	B	C
2	754.36	6.011	0.81
5	1183.74	7.641	0.838
10	1574.38	9.025	0.86
25	2019.37	9.824	0.875
50	2270.67	9.984	0.876
100	2619.36	10.5	0.884
250	3048.22	10.03	0.888

*Intensity $i = A / (t+B)^C$ (mm/t)

* Refer to the City of London Design Specification & Requirements Manual (DS&RM), Section

PRE-DEVELOPMENT 100-YEAR EVENT STORM FLOW

C=	0.37
Time to concentration $t_c =$	19 min
Intensity, i (@ t_c) =	131.4840359 mm/hr
P1 Pre Development Flow, $Q_r = 2.78 * C^{0.75} * A$ =	18.52 l/s
THE ALLOWABLE FLOW FROM THIS SITE IS =	37.54 l/s

PRE-DEVELOPMENT 2-YEAR EVENT STORM FLOW

C=	0.37
Time to concentration $t_c =$	19 min
Intensity, i (@ t_c) =	55.60 mm/hr
P1 Pre Development Flow, $Q_r = 2.78 * C^{0.75} * A$ =	7.84 l/s
THE ALLOWABLE FLOW FROM THIS SITE IS =	15.87 l/s
<i>A=0.089 Ha, C=0.75, Tc=19</i>	

POST-DEVELOPMENT

	Area (m ²)	C	A°C
Total Area:	1369.33		
Building Area:	349.85	0.9	314.87
Asphalt Parking Lot	464.60	0.9	418.14
Landscape/Open:	422.75	0.2	84.55
Sidewalks	132.13	0.9	118.92
		0	0.00
		0	0.00
Totals:	1369.33		936.47
$C_{eq} = \text{Sum}(A^\circ C) / \text{Sum}(A) =$	0.68		

POST-DEVELOPMENT CATCHMENT 201

	Area (m ²)	C	A°C
Total Area:	1000.07		
Building Area:	349.85	0.9	314.87
Asphalt Parking Lot	464.60	0.9	418.14
Landscape/Open:	72.13	0.2	14.43
Sidewalks	113.49	0.9	102.14

POST-DEVELOPMENT CATCHMENT U1+U2

	Area (m ²)	C	A°C
Total Area:	396.26		
Building Area:	0.00	0.9	0.00
Concrete Sidewalks/Entra	18.64	0.9	16.78
Landscape/Open:	350.62	0.2	70.12

Totals: 1000.07 849.57

$C_{eq} = \text{Sum}(A \cdot C) / \text{Sum}(A) =$ 0.85

POST-DEVELOPMENT 100-YEAR EVENT STORM FLOW

C= 0.85
 Time to concentration tc= 12.5 min
 Intensity, $i (@ t_c) = 163.8431472$ mm/hr
 A1 Post Development Flow, $Q_r = 2.78 \cdot C \cdot i \cdot A =$ 38.70 l/s

POST-DEVELOPMENT 2-YEAR EVENT STORM FLOW

C= 0.85
 Time to concentration tc= 12.5 min
 Intensity, $i (@ t_c) = 70.95$ mm/hr
 A1 Post Development Flow, $Q_r = 2.78 \cdot C \cdot i \cdot A =$ 16.76 l/s

THE ALLOWABLE FLOW FROM THIS SITE IS =
 A=0.89 Ha, C=0.75, Tc=11.75

Totals: 369.26 86.90

$C_{eq} = \text{Sum}(A \cdot C) / \text{Sum}(A) =$ 0.24

POST-DEVELOPMENT 100-YEAR EVENT STORM FLOW

C= 0.24
 Time to concentration tc= 19 min
 Intensity, $i (@ t_c) = 131.484$ mm/hr
 U1 Post Development Flow, $Q_r = 2.78 \cdot C \cdot i \cdot A =$ 3.41 l/s

POST-DEVELOPMENT 2-YEAR EVENT STORM FLOW

C= 0.24
 Time to concentration tc= 19 min
 Intensity, $i (@ t_c) = 55.60$ mm/hr
 U1 Post Development Flow, $Q_r = 2.78 \cdot C \cdot i \cdot A =$ 1.44 l/s

RETURN PERIOD OF STORM	PREDEVELOPMENT (201) (l/s)	UNCONTROLLED POST DEVELOPMENT FLOW (U1+U2)	ALLOWABLE POST-DEVELOPMENT FLOWS (201- (U1+U2))
100 YEAR	37.54	3.41	34.13
2 YEAR	15.87	1.44	14.43

FLOW RESTRICTOR CALCULATIONS

Orifice diameter is based on Bernoulli's equation, $Q = C_d \cdot A \cdot (2gH)^{0.5}$
 Rearranging, $A = Q / [C_d \cdot (2gH)^{0.5}]$, where:

Restricted Flow Rate, Q = 14.43 l/s
 Orifice Coefficient, $C_d = 0.63$
 Gravitational Acceleration, g = 9.81 m/s²
 Top of Flooding = N/A
 Invert at Orifice = N/A
 Hydraulic Head on Orifice, H * = 1.2 M 18.6283
 Required Cross-sectional Area, A = 0.004720294
 Required Diameter, $d = ((4 \cdot A) / \pi)^{0.5} = 0.077524518$
 Minimum Orifice diameter = 76 mm
 Therefore use Orifice Restrictor Diameter = 78 mm

MIN ALLOWABLE FLOW DUE TO ORIFICE = 12.06 L/S

AES PARAMETERS FOR INTENSITY DURATION FREQUENCY CURVES

RETURN PERIOD	A, B, C, PARAMETERS		
	A	B	C
2	754.36	6.011	0.81
5	1183.74	7.641	0.838
10	1574.38	9.025	0.86
25	2019.37	9.824	0.875
50	2270.67	9.984	0.876
100	2619.36	10.5	0.884
250	3048.22	10.03	0.888

*Intensity $i=A/(t+B)^C$ (mm/hr)

* Refer to the City of London Design Specification & Requirments Manual (DS&RM), Section

2 YR STORM EVENT

DURATION (MIN)	INTENSITY "i" (mm/hr)
5	108.07
10	79.80
15	64.03
30	41.39
60	25.33
120	15.01
180	10.95

2 YR STORM EVENT

STORAGE CALCULATIONS

A1 INFLOW, Qi $2.78^C \cdot A \cdot i$ (l/s)	Volume in $Qi \cdot t \cdot 60 / 1000$	Design Outflow Qo (l/s)	Surface Outflow Qo (l/s)	Volume OUT $Qo \cdot t \cdot 60 / 1000$ m ³	Difference / Storage m ³
25.52	7.66	14.43	0	4.329	3.33
18.85	11.31	14.43	0	8.658	2.65
15.12	13.61	14.43	0	12.987	0.62
9.77	17.59	14.43	0	25.974	-8.38
5.98	21.54	14.43	0	51.948	-30.41
3.54	25.52	14.43	0	103.896	-78.38
2.59	27.92	14.43	0	155.844	-127.92

5 YR STORM EVENT

Maximum Storage Volume (m³)=

3.33

Post Development Required Storage

5 YR STORM EVENT

DURATION (MIN)	INTENSITY "i" (mm/hr)
5	141.24
10	106.82
15	86.67
30	56.60
60	34.64
120	20.34
180	14.73

STORAGE CALCULATIONS

A1 INFLOW, Qi $2.78^C \cdot A \cdot i$ (l/s)	Volume in $Qi \cdot t \cdot 60 / 1000$	Design Outflow Qo (l/s)	Surface Outflow Qo (l/s)	Volume OUT $Qo \cdot t \cdot 60 / 1000$ m ³	Difference / Storage m ³
33.36	10.01	14.43	0	4.329	5.68
25.23	15.14	14.43	0	8.658	6.48
20.47	18.42	14.43	0	12.987	5.44
13.37	24.06	14.43	0	25.974	-1.91
8.18	29.45	14.43	0	51.948	-22.50
4.80	34.60	14.43	0	103.896	-69.30
3.48	37.57	14.43	0	155.844	-118.27

Maximum Storage Volume (m³)=

6.48

Post Development Required Storage

100 YR STORM EVENT

DURATION (MIN)	INTENSITY "i" (mm/hr)
5	232.24
10	181.39
15	149.56
30	99.36
60	60.87
120	35.32
180	25.28

100 YR STORM EVEI

STORAGE CALCULATIONS

A1 INFLOW, Qi $2.78^C \cdot A \cdot i$ (l/s)	Volume in $Qi \cdot t \cdot 60 / 1000$	Design Outflow Qo (l/s)	Surface Outflow Qo (l/s)	Volume OUT $Qo \cdot t \cdot 60 / 1000$ m ³	Difference / Storage m ³
54.85	16.46	14.43	0	4.329	12.13
42.84	25.70	14.43	0	8.658	17.05
35.32	31.79	14.43	0	12.987	18.80
23.47	42.24	14.43	0	25.974	16.27
14.38	51.75	14.43	0	51.948	-0.19
8.34	60.06	14.43	0	103.896	-43.84
5.97	64.48	14.43	0	155.844	-91.36

Maximum Storage Volume (m³)=

18.80

Post Development Required Storage

PRE-DEVELOPMENT

	Area (m ²)	C	A°C
Total Area:	785.98		
Building Area:	137.12	0.9	123.41
Driveway	0	0.9	0.00
Sidewalk	17.49	0.9	15.74
Front Portch	0	0.9	0.00
Patio	22.12	0.9	19.91
Shed	12.55	0.9	11.30
Landscape/Open:	596.7	0.2	119.34
Totals:	785.98		289.69

$C_{eq} = \text{Sum}(A \cdot C) / \text{Sum}(A) =$

0.37

AES PARAMETERS FOR INTENSITY DURATION FREQUENCY CURVES

RETURN PERIOD	A, B, C, PARAMETERS		
	A	B	C
2	754.36	6.011	0.81
5	1183.74	7.641	0.838
10	1574.38	9.025	0.86
25	2019.37	9.824	0.875
50	2270.67	9.984	0.876
100	2619.36	10.5	0.884
250	3048.22	10.03	0.888

*Intensity $i = A / (t + B)^C$ (mm/h)

* Refer to the City of London Design Specification & Requirements Manual (DS&RM), Section

PRE-DEVELOPMENT 100-YEAR EVENT STORM FLOW

C= 0.37
 Time to concentration $t_c = 19$ min
 Intensity, $i (@ t_c) = 131.484036$ mm/hr
 P1 Pre Development Flow, $Q_r = 2.78 \cdot C \cdot i \cdot A = 10.59$ l/s

PRE-DEVELOPMENT 2-YEAR EVENT STORM FLOW

C= 0.37
 Time to concentration $t_c = 19$ min
 Intensity, $i (@ t_c) = 55.60$ mm/hr
 P1 Pre Development Flow, $Q_r = 2.78 \cdot C \cdot i \cdot A = 4.48$ l/s

DATE: September 03, 2024
 PROJECT #: 24-1011
 CLIENT: Creative Structure
 ADDRESS: 691 Fanshawe Park Road, London
 LOCATION: London, ON. **CATCHMENT 102**

PRE-DEVELOPMENT

	Area (m ²)	C	A*C
Total Area:	583.35		
Building Area:	0	0.9	0.00
Driveway	121.93	0.9	109.74
Sidewalk	13.99	0.9	12.59
Front Portch	7.75	0.9	6.98
Patio	0	0.9	0.00
Shed	0	0.9	0.00
Landscape/Open:	439.68	0.2	87.94
Totals:	583.35		217.24

$C_{eq} = \text{Sum}(A^*C)/\text{Sum}(A) =$ 0.37

AES PARAMETERS FOR INTENSITY DURATION FREQUENCY CURVES

RETURN PERIOD	A, B, C, PARAMETERS		
	A	B	C
2	754.36	6.011	0.81
5	1183.74	7.641	0.838
10	1574.38	9.025	0.86
25	2019.37	9.824	0.875
50	2270.67	9.984	0.876
100	2619.36	10.5	0.884
250	3048.22	10.03	0.888

*Intensity $i = A/(t+B)^C$ (mm/t)

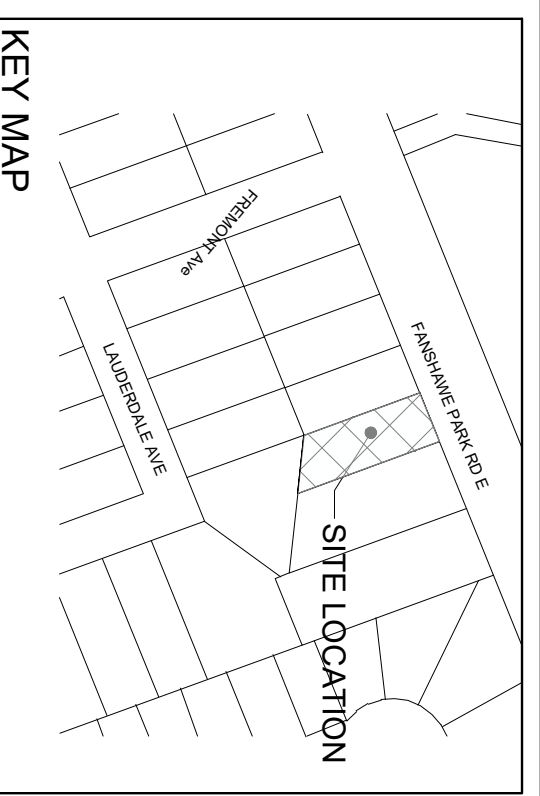
* Refer to the City of London Design Specification & Requirements Manual (DS&RM), Section

PRE-DEVELOPMENT 100-YEAR EVENT STORM FLOW

C= 0.37
 Time to concentration $t_c = 19$ min
 Intensity, i (@ t_c) = 131.4840359 mm/hr
 P1 Pre Development Flow, $Q_r = 2.78 * C * i * A =$ 7.94 l/s

PRE-DEVELOPMENT 2-YEAR EVENT STORM FLOW

C= 0.37
 Time to concentration $t_c = 19$ min
 Intensity, i (@ t_c) = 55.60 mm/hr
 P1 Pre Development Flow, $Q_r = 2.78 * C * i * A =$ 3.36 l/s



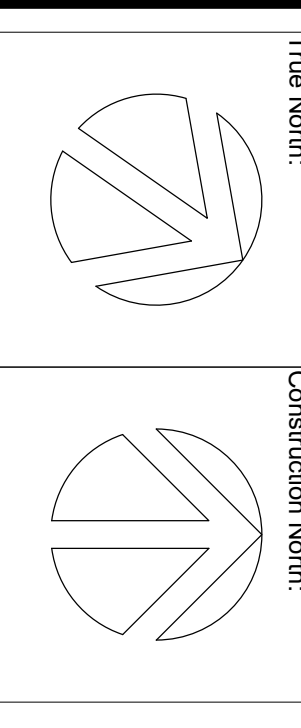
KEY MAP



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The Contractor shall verify and be responsible for a dimensions. DO NOT scale the drawings - any errors or omissions are the responsibility of the Contractor. Prototek Engineering Associates Inc. Reproduction or use for any purpose other than that authorized by Prototek Engineering Associates Inc. is forbidden.

True North:



Notes

These drawings have been prepared based on information provided by others. Prototek Engineering Associates Inc. has not verified the accuracy and/or completeness of this information and shall not be responsible for any errors or omissions which may be incorporated herein as a result.

R2	REES	NHA	NHA	2024SEP03
R1	REES	NHA	NHA	2024MAY02
Revision:	Dwn.	Chkd.	Dsgn.	Date

ISSUED FOR CLIENT REVIEW	2024SER03
ISSUED FOR CLIENT REVIEW	2024MAY02
Issued for:	Date
Permit/Seal	Permit/Seal

Client Information:
CANADIAN BUSINESS CREATIVITY
891 FANSHAWE PARK ROAD, EAST
LONDON, ON

Project Information:
891 FANSHAWE - STACKED TOWNHOUSES
891 FANSHAWE PARK ROAD, EAST
LONDON, ON

Drawing Title:
CONCEPTUAL PROPOSED GRADING AND
SERVICING PLAN

Submission Date:
2024SEP03

Project No. 24-1011
Scale AS NOTED

Drawing No. C101
Sheet # 2
Revision 000

TYPICAL LEGEND

- PH FIRE HYDRANT
- WV WATER VALVE
- GV GAS VALVE
- SM NEW STORM MANHOLE
- SM EXISTING STORM MANHOLE
- SM NEW SANITARY MANHOLE
- SM EXISTING SANITARY MANHOLE
- CB NEW CATCH BASIN
- CB EXISTING CATCH BASIN
- CBM NEW STORM CATCH BASIN MANHOLE
- CBM EXISTING STORM CATCH BASIN MANHOLE
- EH DIRECTION OF SURFACE WATER FLOW
- EW DIRECTION OF SWALE WATER FLOW
- EE EXISTING ELEVATION
- PE PROPOSED ELEVATION
- W EXISTING WATERMAIN
- W NEW WATERMAIN
- ST NEW STORM WATER PIPE
- ST EX. STORM WATER PIPE
- SP NEW SANITARY PIPE
- UP LIGHT POLE
- UP UTILITY POLE
- BP BOLLARD
- FC FENCE
- PS PEDESTAL
- PE LIMIT OF WORK
- UE UNDERGROUND ELECTRICAL CABLE LINE
- CF SILT FENCE
- PL PROPERTY LINE
- UL UTILITY LINE
- PI PIPE INSULATION
- PO TOP OF BANK
- PF PIPE FLOW DIRECTION
- SB SUB-CATCHMENT BOUNDARY
- OF OVERLAND FLOW ROUTE
- XXX CATCHMENT NAME
- XXX RUNOFF COEFFICIENT
- HP HIGH POINT

NOTES

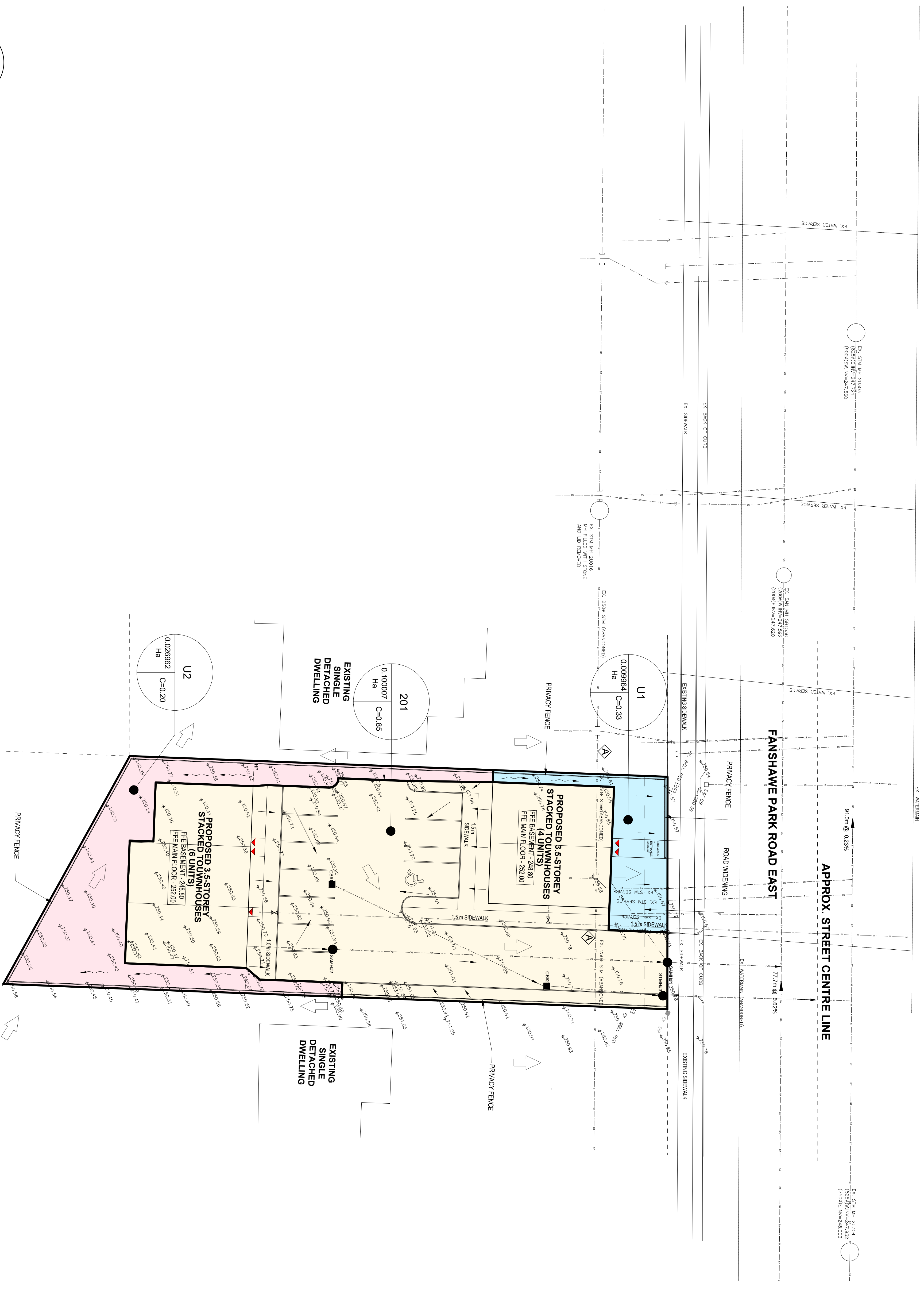
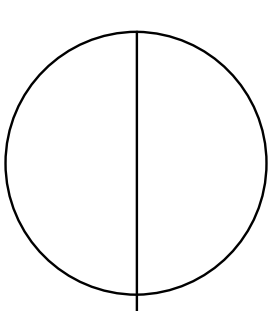
- EXIST. STORM LINES (ABANDONED) WITHIN THE PROPERTY LINE TO BE REMOVED

CATCHMENT TABLE

LANDSCAPE	LANDSCAPE	LANDSCAPE
LANDSCAPE	81.00 m ²	81.00 m ²
SIDEWALKS	18.64 m ²	18.64 m ²
TOTAL AREA OF U1	99.64 m ²	99.64 m ²
LANDSCAPE	72.13 m ²	72.13 m ²
ASPHALT PARKING LOT	464.60 m ²	464.60 m ²
BUILDING 4 UNITS	147.60 m ²	147.60 m ²
BUILDING 6 UNITS	202.25 m ²	202.25 m ²
SIDEWALKS	113.49 m ²	113.49 m ²
TOTAL AREA OF 201	1000.07 m ²	1000.07 m ²
LANDSCAPE	269.62 m ²	269.62 m ²
TOTAL AREA OF U2	269.62 m ²	269.62 m ²
OVERALL TOTAL AREA	1369.33 m ²	1369.33 m ²

*NOTE: ALL ROOF DRAINAGE FOR BOTH UNITS TO BE DIRECTED TOWARDS PARKING LOT VIA DOMINSPOUT

CONCEPTUAL PROPOSED GRADING AND SERVICING PLAN

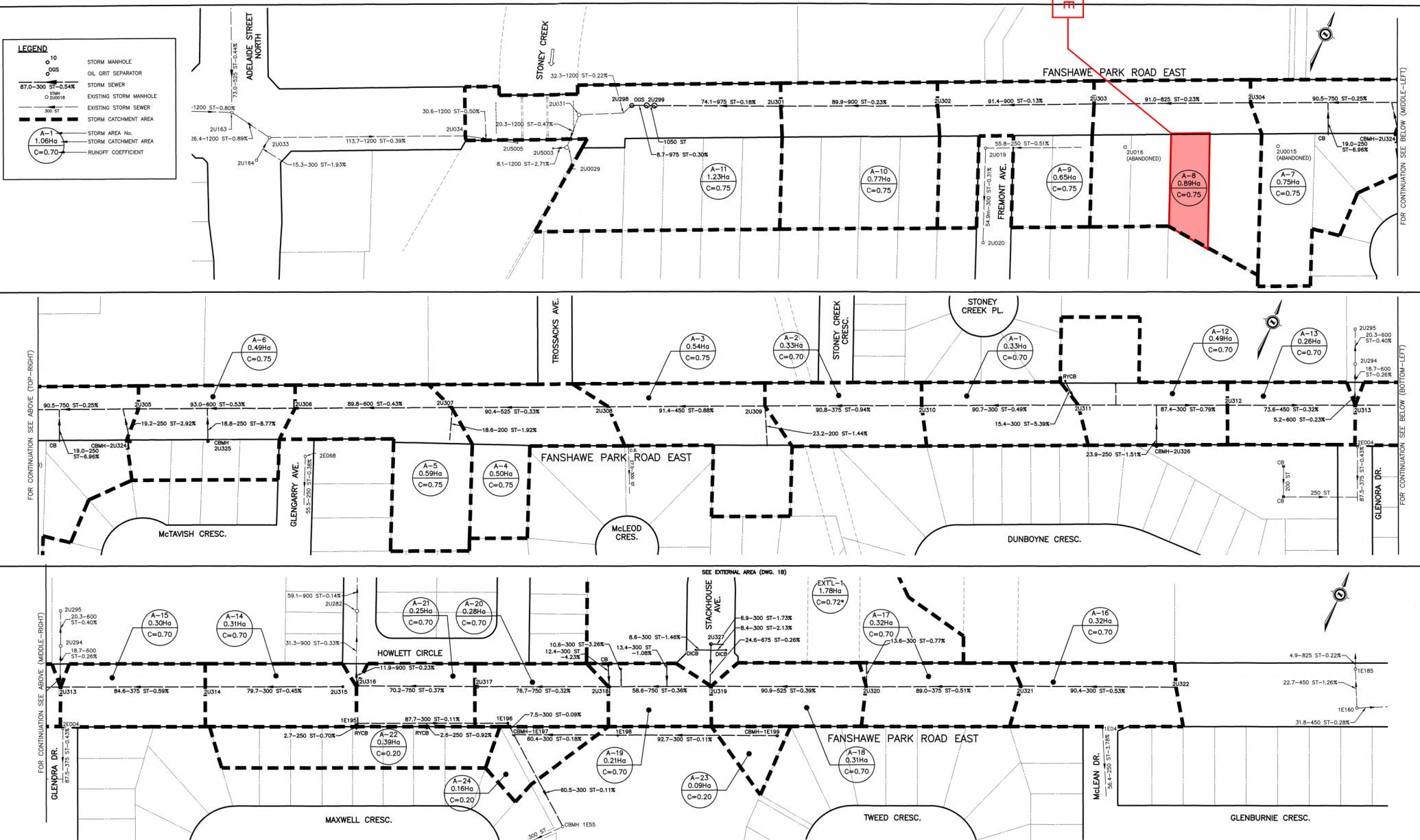


CONCEPTUAL PROPOSED GRADING AND SERVICING PLAN

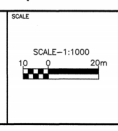
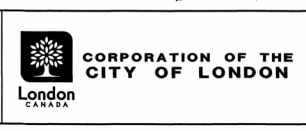
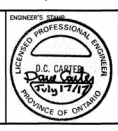
APPENDIX "B"

SUBJECT SITE

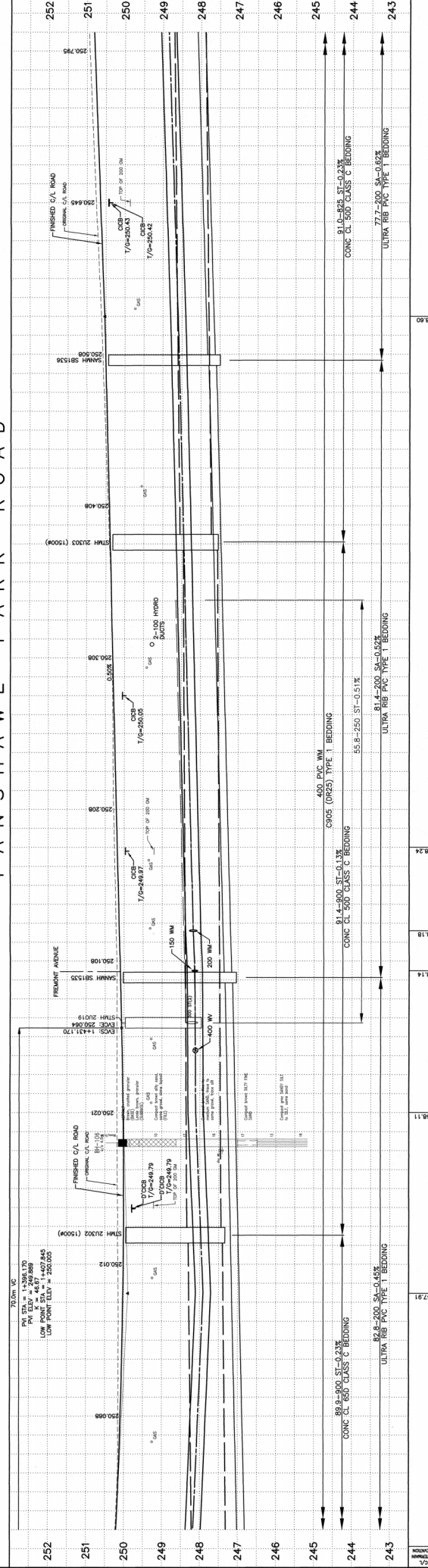
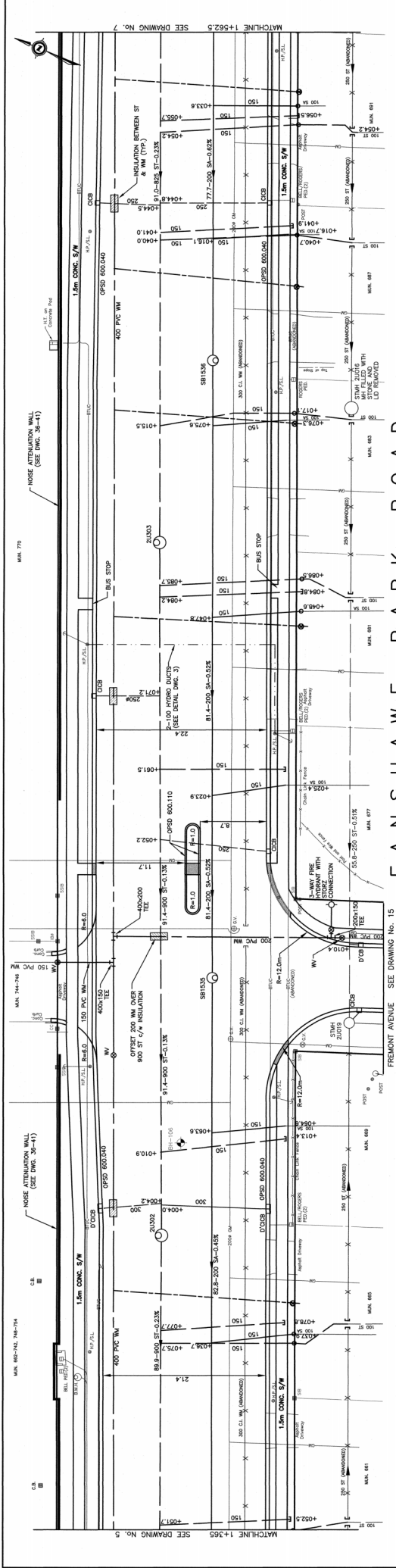
LEGEND	
	STORM MANHOLE
	OIL GRIT SEPARATOR
	STORM SEWER
	EXISTING STORM MANHOLE
	EXISTING STORM SEWER
	STORM CATCHMENT AREA
	STORM AREA No. 1.06Ha C=0.70
	STORM CATCHMENT AREA
	RUNOFF COEFFICIENT



EXISTING SERVICES	DRAWING #	SOURCE	DATE	AS CONSTRUCTED SERVICES	COMPLETION	DETAILS	No.	REVISIONS	DATE	CONSULTANT	CONSULTANT OR DIVISION
	DESIGN	DCS/RJS	1.	ISSUED FOR S&B REVIEW	MARCH 27/15		1.			AECOM	
	DRAWN BY	RJS	2.	ISSUED FOR S&B REVIEW	APRIL 27/15		2.			AECOM	
	CHECKED	JOK	3.	ISSUED FOR 100% REVIEW	NOVEMBER 4/15		3.			AECOM	
	APPROVED	DCS	4.	ISSUED FOR TENDER	DECEMBER 22/15		4.			AECOM	
	DATE	NOV. 2014	5.	RECORD DRAWING	FEBRUARY 24/17		5.			AECOM	



FANSHAWE PARK ROAD IMPROVEMENTS ADELAIDE STREET TO McLEAN DRIVE		PROJECT NO. 60333430
STORM AREA PLAN		SHEET NO. 1A
		27101



EXISTING SERVICES	DRAWING #	SOURCE	DATE	AS CONSTRUCTED SERVICES	COMPLETION	DETAILS	NO.	REVISIONS	DATE	CONSULTANT	CONSULTANT OR DRAWN
SPWATER MAINS, POSE & SPS	5011	FEB 1977		1. REVISION FOR NEW BEDDING	NOV 2014	SECTION 1	1		14-03-03	ACOM	
				2. REVISION FOR NEW BEDDING	NOV 2014	SECTION 2	2		14-03-03	ACOM	
				3. REVISION FOR NEW BEDDING	NOV 2014	SECTION 3	3		14-03-03	ACOM	
				4. REVISION FOR NEW BEDDING	NOV 2014	SECTION 4	4		14-03-03	ACOM	
				5. REVISION FOR NEW BEDDING	NOV 2014	SECTION 5	5		14-03-03	ACOM	
				6. REVISION FOR NEW BEDDING	NOV 2014	SECTION 6	6		14-03-03	ACOM	
				7. REVISION FOR NEW BEDDING	NOV 2014	SECTION 7	7		14-03-03	ACOM	
				8. REVISION FOR NEW BEDDING	NOV 2014	SECTION 8	8		14-03-03	ACOM	
				9. REVISION FOR NEW BEDDING	NOV 2014	SECTION 9	9		14-03-03	ACOM	
				10. REVISION FOR NEW BEDDING	NOV 2014	SECTION 10	10		14-03-03	ACOM	

FANSHAWE PARK ROAD

SEE DRAWING No. 15

FREMONT AVENUE SEE DRAWING No. 15

FANSHAWE PARK ROAD

SEE DRAWING No. 7

MATCHLINE 1+365

MATCHLINE 1+562.5

SCALE

HORIZONTAL: 1:250

VERTICAL: 1:50

CORPORATION OF THE CITY OF LONDON

LONDON

ENGINEER

PROJECT NO.

6

FANSHAWE PARK ROAD IMPROVEMENTS

ADJACENT STREET TO INCLINE DRIVE

PROJECT NO. 603334.30

SHEET NO. 27108

FROM 75m WEST OF FREMONT AVENUE TO 125m EAST OF FREMONT AVENUE